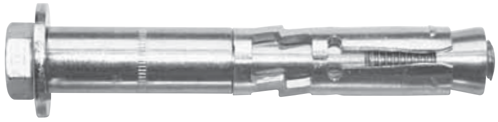
SPIT TRIGA Z-A4

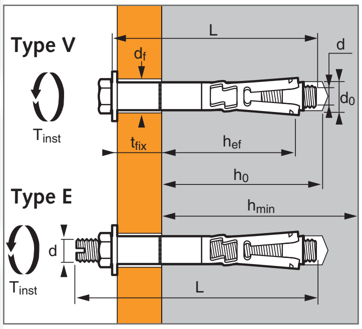
Stainless steel



High security, high performance fixing

Technical data

SPIT TRIGA Z	Min. anchor depth (mm) h_{ef}	Max. thick of part to be fixed (mm) t_{fix}	Min thick of base material (mm) h_{min}	Ø thread (mm) d	Drilling depth (mm) h_0	Ø drill bit (mm) d_0	Ø clearance (mm) d_f	Total anchor length (mm) L	Max. tighten torque (Nm) T_{ins}	Code
V6-10/10	50	10	100	M6	70	10	12	70	10	050694
V8-12/10		10						80		050595
V8-12/30	60	30	120	M8	80	12	14	100	25	050596
E8-12/45		45						124		050598
V10-15/25	70	25	140	M10	90	15	17	115	50	050601
E10-15/45		45						139		050604
V12-18/25		25						120		050605
E12-18/15	80	15	160	M12	105	18	20	122	80	050606
E12-18/45		45						152		050608
E16-24/25	95	25	200	M16	130	24	26	157	120	052940



Anchor mechanical properties

		M6	M8	M10	M12	M16
Type V						
f_{uk} (N/mm²)	Min. tensile strength	800	800	800	800	800
f_{yk} (N/mm²)	Yield strength	600	600	600	600	600
$M^0_{rk,s}$ (Nm)	Characteristic bending moment	12,2	30,0	59,8	104,8	266,4
M (Nm)	Recommended bending moment	5,8	12,4	24,8	43,5	110,7
Type E						
f_{uk} (N/mm²)	Min. tensile strength	700	700	700	700	700
f_{yk} (N/mm²)	Yield strength	350	350	350	350	350
$M^0_{rk,s}$ (Nm)	Characteristic bending moment	10,6	26,2	52,3	91,7	233,1
M (Nm)	Recommended bending moment	4,4	10,9	21,8	38,2	97,1
Type V and Type E						
$S_{eq,V}$ (mm²)	Equivalent stressed cross-section bolt version	39,2	76,1	108,8	175,3	335,1
$S_{eq,E}$ (mm²)	Equivalent stressed cross-section threaded stud version	35,2	61,8	82,0	104,1	183,3
W_{el} (mm³)	Elastic section modulus	12,7	31,2	62,3	109,2	277,5

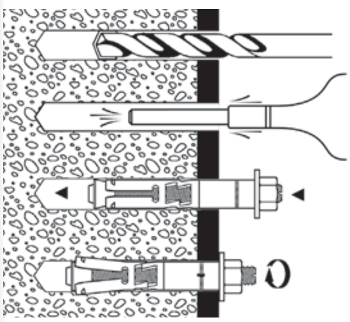
APPLICATION

- Safety critical loads
- Overhead crane rails
- Steel columns and walkways
- Wall plates
- Safety rails

MATERIAL

- **Bolt:** class 80 - NF EN ISO 3506-1
- **Threaded stud:** class 70 - NF E 25100-0
- **Nut:** class 80 - NF E 25100-4
- **Washer:** X5CrNiMo 17-12-2
- **Sleeve:** X2CrNiMo 17-12-2
- **Expansion cone:** X2CrNiMo 17-12-2
- **Expansion sleeve:** X2CrNiMo 17-12-2

INSTALLATION



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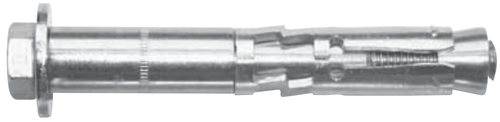

The loads specified on this page allow judging the product's performances, but cannot be used for the designing.
The data given in the pages "CC method" have to be applied.

Ultimate ($N_{Ru,m}$, $V_{Ru,m}$) / characteristic loads (N_{Rk}, V_{Rk}) in kN

Mean Ultimate loads are derived from test results in admissible service conditions, and characteristic loads are statistically determined.

TENSILE

Anchor size	M6	M8	M10	M12	M16
Non cracked concrete (C20/25)					
h_{ef}	*50*	*60*	*70*	*80*	*95*
$N_{Ru,m}$	16,7	22,4	38,7	41,3	64,2
N_{Rk}	16	17	26	28	56
Cracked concrete (C20/25)					
h_{ef}	*50*	*60*	*70*	*80*	*95*
$N_{Ru,m}$	14,8	25,2	33,8	40,4	55,9
N_{Rk}	11	21	25	28,8	38

SHEAR

Anchor size	M6	M8	M10	M12	M16
Cracked and non cracked concrete (C20/25)					
$V_{Ru,m}$ (Type V)	26,8	37,6	70,1	67,4	140,7
V_{Rk}	21,6	31,3	58,4	60,1	117,2
$V_{Ru,m}$ (Type E)	17,5	22,9	37,7	49,9	101,5
V_{Rk}	14,6	19,1	31,4	41,5	84,6

Design Loads (N_{Rd}, V_{Rd}) for one anchor without edge or spacing influence in kN

$$N_{Rd} = \frac{N_{Rk}^{*}}{\gamma_{Mc}}$$

$$V_{Rd} = \frac{V_{Rk}^{*}}{\gamma_{Ms}}$$

*Derived from test results

TENSILE

Anchor size	M6	M8	M10	M12	M16
Non cracked concrete (C20/25)					
h_{ef}	*50*	*60*	*70*	*80*	*95*
N_{Rd}	10,7	11,6	17,3	18,5	31,0
Cracked concrete (C20/25)					
h_{ef}	*50*	*60*	*70*	*80*	*95*
N_{Rd}	7,3	14,0	16,7	19,2	21,1

γ_{Mc} = 1,5 for M8-M12 and γ_{Mc} = 1,8 for M16

SHEAR

Anchor size	M6	M8	M10	M12	M16
Cracked and non cracked concrete (C20/25)					
V_{Rd} (Type V)	16,2	23,6	36,9	45,2	88,1
V_{Rd} (Type E)	7,3	9,5	15,7	20,8	42,3

γ_{Ms} = 1,33 for Type V and γ_{Ms} = 2,0 for Type E

Recommended loads (N_{rec}, V_{rec}) for one anchor without edge or spacing influence in kN

$$N_{Rec} = \frac{N_{Rk}^{*}}{\gamma_{M}.\gamma_{F}}$$

$$V_{Rec} = \frac{V_{Rk}^{*}}{\gamma_{M}.\gamma_{F}}$$

*Derived from test results

TENSILE

Anchor size	M6	M8	M10	M12	M16
Non cracked concrete (C20/25)					
h_{ef}	*50*	*60*	*70*	*80*	*95*
N_{Rec}	7,7	8,3	12,3	13,2	22,1
Cracked concrete (C20/25)					
h_{ef}	*50*	*60*	*70*	*80*	*95*
N_{Rec}	5,2	10,0	11,9	13,7	15,1

γ_F = 1,4 ; γ_{Mc} = 1,5 for M8-M12 and γ_{Mc} = 1,8 for M16

SHEAR

Anchor size	M6	M8	M10	M12	M16
Cracked and non cracked concrete (C20/25)					
V_{Rec} (Type V)	11,6	16,8	26,4	32,2	63,0
V_{Rec} (Type E)	5,2	6,8	11,2	14,8	30,2

γ_F = 1,4 ; γ_{Ms} = 1,33 for Type V and γ_{Ms} = 2,0 for Type E

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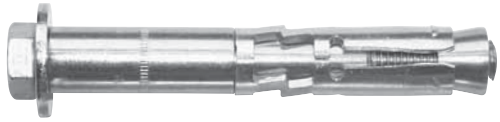
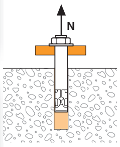


SPIT CC- Method

TENSILE in kN

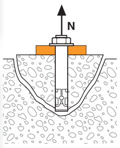


Pull-out resistance

$$N_{Rd,p} = N^{O}_{Rd,p} . f_b$$

$N^0_{Rd,p}$ Anchor size	Design pull-out resistance M6	M8	M10	M12	M16
Non cracked concrete					
h_{ef}	*50*	*60*	*70*	*80*	*95*
$N^0_{Rd,p}$ (C20/25)	-	10,6	13,3	16,6	-
Cracked concrete					
h_{ef}	*50*	*60*	*70*	*80*	*95*
$N^0_{Rd,p}$ (C20/25)	3,3	6	10,6	-	-

γ_{Mc} = 1,5 for M6-M12

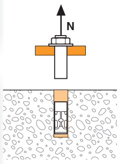


Concrete cone resistance

$$N_{Rd,c} = N^{O}_{Rd,c} . f_b . \Psi_s . \Psi_{c,N}$$

$N^0_{Rd,c}$ Anchor size	Design cone resistance M6	M8	M10	M12	M16
Non cracked concrete					
h_{ef}	*50*	*60*	*70*	*80*	*95*
$N^0_{Rd,c}$ (C20/25)	11,9	15,6	19,7	24,0	25,9
Cracked concrete					
h_{ef}	*50*	*60*	*70*	*80*	*95*
$N^0_{Rd,c}$ (C20/25)	8,5	11,2	14,1	17,2	18,5

γ_{Mc} = 1,5 for M6-M12 and γ_{Mc} = 1,8 for M16

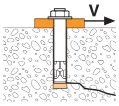


Steel resistance

$N_{Rd,s}$ Anchor size	Steel design tensile resistance M6	M8	M10	M12	M16
$N_{Rd,s}$ (Type V)	10,0	18,2	28,8	42,0	78,9
$N_{Rd,s}$ (Type E)	5,8	10,6	16,8	24,4	45,9

γ_{Ms} = 1,6 for Type V and γ_{Ms} = 2,4 for Type E

SHEAR in kN

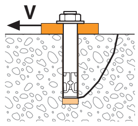


Concrete edge resistance

$$V_{Rd,c} = V^{O}_{Rd,c} . f_b . f_{\beta,V} . \Psi_{S-C,V}$$

$V^0_{Rd,c}$ Anchor size	Design concrete edge resistance at minimum edge distance (C_{min}) M6	M8	M10	M12	M16
Non cracked concrete					
h_{ef}	*50*	*60*	*70*	*80*	*95*
C_{min}	50	60	70	80	100
S_{min}	100	100	160	200	220
$V^0_{Rd,c}$ (C20/25)	3,4	4,9	6,8	9,3	13,6
Cracked concrete					
h_{ef}	*50*	*60*	*70*	*80*	*95*
C_{min}	50	60	70	80	100
S_{min}	100	100	160	200	220
$V^0_{Rd,c}$ (C20/25)	2,4	3,5	4,8	6,6	9,7

γ_{Mc} = 1,5

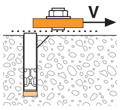


Pryout failure

$$V_{Rd,cp} = V^{0}_{Rd,cp} . f_b . \Psi_s . \Psi_{c,N}$$

$V^0_{Rd,cp}$ Anchor size	Design pryout resistance M6	M8	M10	M12	M16
Non cracked concrete					
h_{ef}	*50*	*60*	*70*	*80*	*95*
$V^0_{Rd,cp}$ (C20/25)	11,9	31,2	39,4	48,1	62,2
Cracked concrete					
h_{ef}	*50*	*60*	*70*	*80*	*95*
$V^0_{Rd,cp}$ (C20/25)	8,5	22,3	28,1	34,3	44,4

γ_{Mcp} = 1,5



Steel resistance

$V_{Rd,s}$ Anchor size	Steel design shear resistance M6	M8	M10	M12	M16
Cracked and non cracked concrete					
$V_{Rd,s}$ (Type V)	16,2	23,6	36,9	45,2	88,2
$V_{Rd,s}$ (Type E)	6,3	8,3	13,6	20,7	40,7

γ_{Ms} = 1,33 for Type V and γ_{Ms} = 2,0 for Type E

$$N_{Rd} = \min(N_{Rd,p} ; N_{Rd,c} ; N_{Rd,s})$$
$$\beta_N = N_{Sd} / N_{Rd} \leq 1$$

$$V_{Rd} = \min(V_{Rd,c} ; V_{Rd,cp} ; V_{Rd,s})$$
$$\beta_V = V_{Sd} / V_{Rd} \leq 1$$

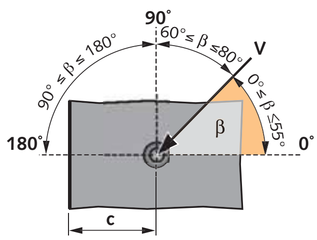
$$\beta_N + \beta_V \leq 1,2$$

f_B INFLUENCE OF CONCRETE

Concrete class	f_B	Concrete class	f_B
C25/30	1,1	C40/50	1,41
C30/37	1,22	C45/55	1,48
C35/45	1,34	C50/60	1,55

$f_{\beta,V}$ INFLUENCE OF SHEAR LOADING DIRECTION

Angle β [°]	$f_{\beta,V}$
0 to 55	1
60	1,1
70	1,2
80	1,5
90 to 180	2



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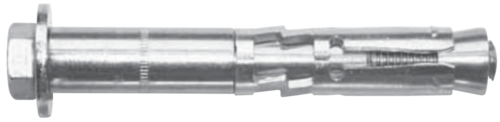
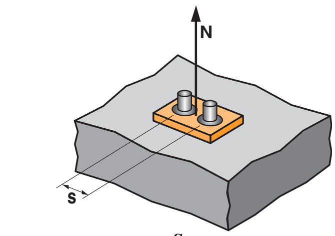
Stainless steel



SPIT CC- Method

Ψ_s INFLUENCE OF SPACING FOR CONCRETE CONE RESISTANCE IN TENSILE LOAD

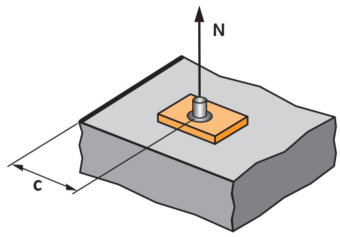


$$\Psi_S = 0,5 + \frac{s}{6.h_{ef}}$$

S_{min} < S < $S_{cr,N}$
$S_{cr,N}$ = 3.h_{ef}
Ψ_S must be used for each spacing influenced the anchors group.

SPACING S	Reduction factor Ψ_s Cracked and non-cracked concrete				
	M6	M8	M10	M12	M16
50	0,67				
60	0,70	0,67			
70	0,73	0,69	0,67		
80	0,77	0,72	0,69	0,67	
100	0,83	0,78	0,74	0,71	0,67
125	0,92	0,85	0,80	0,76	0,71
150	1,00	0,92	0,86	0,81	0,75
180		1,00	0,93	0,88	0,80
210			1,00	0,94	0,85
240				1,00	0,90
300					1,00

$\Psi_{c,N}$ INFLUENCE OF EDGE FOR CONCRETE CONE RESISTANCE IN TENSILE LOAD

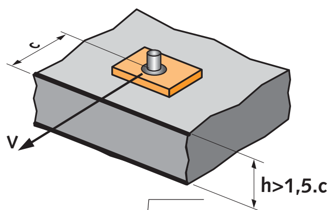


$$\Psi_{c,N} = 0,25 + 0,5.\frac{c}{h_{ef}}$$

C_{min} < C < $C_{cr,N}$
$C_{cr,N}$ = 1,5.h_{ef}
$\Psi_{c,N}$ must be used for each distance influenced the anchors group.

EDGE C	Reduction factor $\Psi_{c,N}$ Cracked and non-cracked concrete				
	M6	M8	M10	M12	M16
50	0,75				
60	0,85	0,75			
70	0,95	0,83	0,75		
80	1,00	0,92	0,82	0,75	
90		1,00	0,89	0,81	
100			0,96	0,88	0,75
120				1,00	0,85
150					1,00

$\Psi_{s-c,V}$ INFLUENCE OF SPACING AND EDGE DISTANCE FOR CONCRETE EDGE RESISTANCE IN SHEAR LOAD

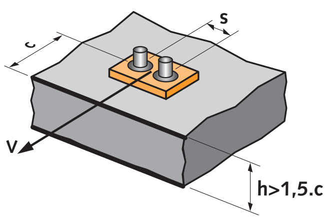


$$\Psi_{s-c,V} = \frac{c}{c_{min}} . \sqrt{\frac{c}{c_{min}}}$$

For single anchor fastening

Factor $\Psi_{s-c,V}$
Cracked and non-cracked concrete

$\frac{C}{C_{min}}$	1,0	1,2	1,4	1,6	1,8	2,0	2,2	2,4	2,6	2,8	3,0	3,2
$\Psi_{s-c,V}$	1,00	1,31	1,66	2,02	2,41	2,83	3,26	3,72	4,19	4,69	5,20	5,72

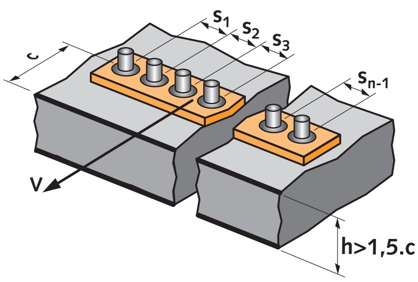


$$\Psi_{s-c,V} = \frac{3.c + s}{6.c_{min}} . \sqrt{\frac{c}{c_{min}}}$$

For 2 anchors fastening

Factor $\Psi_{s-c,V}$
Cracked and non-cracked concrete

$\frac{S}{C_{min}}$ \ $\frac{C}{C_{min}}$	1,0	1,2	1,4	1,6	1,8	2,0	2,2	2,4	2,6	2,8	3,0	3,2
1,0	0,67	0,84	1,03	1,22	1,43	1,65	1,88	2,12	2,36	2,62	2,89	3,16
1,5	0,75	0,93	1,12	1,33	1,54	1,77	2,00	2,25	2,50	2,76	3,03	3,31
2,0	0,83	1,02	1,22	1,43	1,65	1,89	2,12	2,38	2,63	2,90	3,18	3,46
2,5	0,92	1,11	1,32	1,54	1,77	2,00	2,25	2,50	2,77	3,04	3,32	3,61
3,0	1,00	1,20	1,42	1,64	1,88	2,12	2,37	2,63	2,90	3,18	3,46	3,76
3,5		1,30	1,52	1,75	1,99	2,24	2,50	2,76	3,04	3,32	3,61	3,91
4,0			1,62	1,86	2,10	2,36	2,62	2,89	3,17	3,46	3,75	4,05
4,5				1,96	2,21	2,47	2,74	3,02	3,31	3,60	3,90	4,20
5,0					2,33	2,59	2,87	3,15	3,44	3,74	4,04	4,35
5,5						2,71	2,99	3,28	3,71	4,02	4,33	4,65
6,0						2,83	3,11	3,41	3,71	4,02	4,33	4,65



For 3 anchors fastening and more

$$\Psi_{s-c,V} = \frac{3.c + s_1 + s_2 + s_3 + ... + s_{n-1}}{3.n.c_{min}} . \sqrt{\frac{c}{c_{min}}}$$